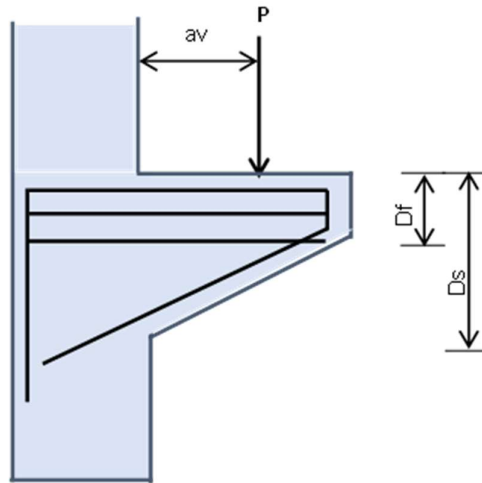


### Corbel design

The basic principle of the design is “**strut-and-tie**” method similar to truss.



The requirement of the corbel is the ratio of  $r = a_v/d$ . The ratio shall be less than 2. Where  $d$  = effective overall depth (Ds minus effective cover). However, ratio  $r = \frac{a_v}{d} \leq 0.6$  in any case.

Again, effective overall depth shall be such that  $\tau_m = \tau_c * \left(\frac{2d}{a_v}\right)$  where  $\tau_c$  = design shear strength of the concrete, depends on the grade of concrete and shall not be greater than  $0.8 \sqrt{f_{ck}}$  or  $5 \text{ N/mm}^2$

The dimensioning of the bracket shall be such that bearing pressure on concrete shall be

- i. Bearing with no padding material  $0.4f_{ck}$ .
- ii. Bearing in cement mortar  $0.6f_{ck}$  (Bedded bearing)
- iii. Bearing on steel plate cast into member  $0.8f_{ck}$ .
- iv. Flexible pads  $0.5f_{ck}$ .

Check for the bearing area

$$\sigma_b = 0.8f_{ck}$$

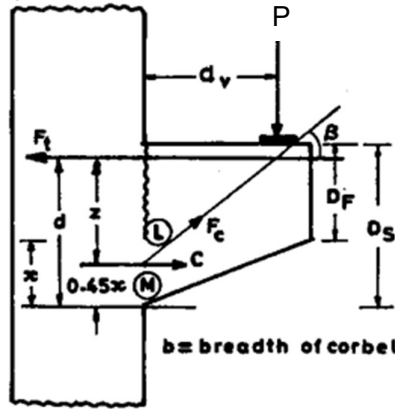
Bearing area

$$A_b = \frac{P * 1000}{\sigma_b}$$

And length of bearing plate

$$l_b = \frac{A_b}{b}$$

Where  $b$  = width of the corbel



Using strut-and-tie method

$$P = F_c \sin(\beta)$$

And  $\sin(\beta) = \frac{z}{\sqrt{a_v^2 + z^2}}$  and therefore

$$F_c = P * \frac{\sqrt{a_v^2 + z^2}}{z}$$

Similarly,

$$F_t = F_c * \cos(\beta)$$

$$F_t = P * \frac{a_v}{z}$$

Where  $z = (d - .45x)$  and hence  $x = \frac{d-z}{0.45}$

Calculation of k

$$k = \frac{P * 1000}{0.87 f_{ck} b d}$$

$$\text{And therefore } z = \frac{\left( \frac{r}{r+k} + \sqrt{\left( \frac{r}{r+k} \right)^2 - 4 * \left( \frac{k}{r+k} \right) \left( \frac{a}{d} \right)^2} \right)}{2} * d$$

Reinforcement design

Calculation of main steel

1. Calculation of strain at the steel

$$\epsilon_s = \epsilon_c * \frac{d - x}{x}$$

And  $\epsilon_c = 0.0035$

$$f_s = \epsilon_s E_s$$

If  $f_s > 0.87 f_y$ , then  $f_s = 0.87 f_y$  otherwise  $f_s = \epsilon_s E_s$  where  $E_s = 2 \times 10^5 \text{ N/mm}^2$  (Young's modulus of steel). For  $\epsilon_s$  follow the stress-strain curve of reinforcement (Fig.23 of IS 456:2000 or Table A of SP16 )

Area of main steel

$$A_{st} = \frac{F_t}{f_s}$$

Check for minimum reinforcement

$$A_{min} = 0.4\% \text{ of } bd$$

Horizontal shear reinforcement shall be

$$A_{su} = \frac{A_{st}}{2}$$



*Corbels in precast construction*

**Design of corbel as per IS 456**

Project: Sankhuwa Hydropower Project

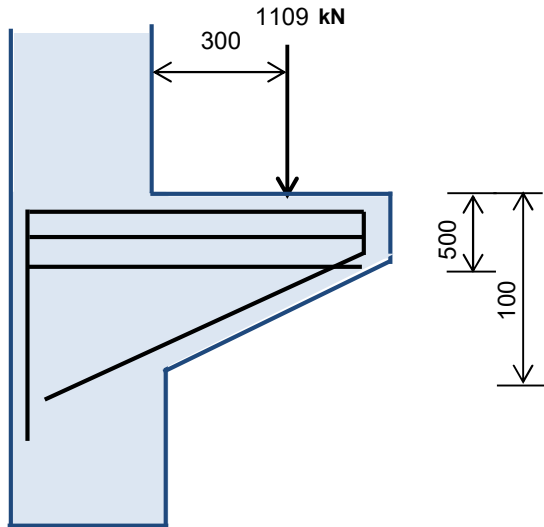
Date: 2/1/2023

Prepared by:

Checked by:

Approved by:

INPUT		Symbol	Value	Unit	OUTPUT			
Design parameters					Size of corbel			
Grade of Concrete			M25		Width		1400	mm
Characteristic Strength of Concrete		$f_{ck}$	25	N/mm <sup>2</sup>	Depth	At bracket	1000	mm
Grade of Steel for main bar		$f_y$	500	N/mm <sup>2</sup>		At end	500	mm
Grade of steel for shear reinforcement		$f_s$	250	N/mm <sup>2</sup>	Reinforcement			
Types of bars			Deformed		Main bar		Dia, mm	Nos
Clear Cover		$d$	40	mm			25	1
Modulus of Elasticity of Steel		$E_s$	200000	N/mm <sup>2</sup>	Shear bar	Dia,mm	Legs	Spacing,mm
							12	4
FACTORED LOAD								
Vertical load		$F_v$	1109	kNm				
Distance of load from support		$a_v$	300	mm				
DIMENSIONING OF CORBEL								
Width of corbel			1400	mm				
Length of bearing pad			40	mm				
Depth of bracket at support			1000	mm				
Depth of corbel at far end			500	mm				
Check for depth								
At bracket			1000.0	mm				
Far end			500.0	mm				
Check for strut ratio $a_v/d$			OK					
DESIGN CALCULATION								
Tensile force		$F_t$	388.0	kN				
Compressive force		$F_c$	1174.5	kN				



<b>REINFORCEMENT DESIGN</b>	Main reinforcement	Shear reinforcement	
Area of bar	5376.00	2688.00	mm <sup>2</sup>
Diameter	<b>25</b>	<b>12</b>	mm
Number of legs for shear bar		4	Nos
Number/Spacing	11	160.0	Nos/mm
Area of bar provided	5399.61		
Percentage of bar	0.41		%
Shear capacity of corbel		5043.28	kN
Check for shear capacity		OK	